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Yield Line Analysis for Slabs

23.1 INTRODUCTION

Most concrete slabs are designed for moments found by the methods described in Chapters 12 and 13. These methods are based essentially upon elastic theory. On the other hand, reinforcement for slabs is calculated by strength methods that account for the actual inelastic behavior of members at the factored load stage. A corresponding contradiction exists in the process by which beams and frames are analyzed and designed, as was discussed in Section 11.9, and the concept of limit, or plastic, analysis of reinforced concrete was introduced. Limit analysis not only eliminates the inconsistency of combining elastic analysis with inelastic design but also accounts for the reserve strength characteristic of most reinforced concrete structures and permits, within limits, an arbitrary readjustment of moments found by elastic analysis to arrive at design moments that permit more practical reinforcing arrangements.

For slabs, there is still another good reason for interest in limit analysis. The elasticity-based methods of Chapter 13 are restricted in important ways. Slab panels must be square or rectangular. They must be supported along two opposite sides (one-way slabs), two pairs of opposite sides (two-way edge-supported slabs), or by a fairly regular array of columns (flat plates and related forms). Loads must be uniformly distributed, at least within the bounds of any single panel. There can be no large openings. But in practice, many slabs do not meet these restrictions. Answers are needed, for example, for round or triangular slabs, slabs with large openings, slabs supported on two or three edges only, and slabs carrying concentrated loads. Limit analysis provides a powerful and versatile tool for treating such problems.

It was evident from the discussion of Section 11.9 that full plastic analysis of a continuous reinforced concrete beam or frame would be tedious and time-consuming because of the need to calculate the rotation requirement at all plastic hinges and to check rotation capacity at each hinge to ensure that it is adequate. Consequently, for beams and frames, the very simplified approach to plastic moment redistribution of ACI Code 6.6.5 is used. However, for slabs, which typically have tensile reinforcement ratios much below the balanced value and consequently have large rotation capacity, it can be safely assumed that the necessary ductility is present. Practical methods for the plastic analysis of slabs are thus possible and have been developed. *Yield line theory*, presented in this chapter, is one of these. Although the ACI Code contains no specific provisions for limit or plastic analysis of slabs, ACI Code 1.1.0 permits use of "any system of design or construction," the adequacy of which has been shown by successful use, analysis, or tests, and ACI Commentary 8.2.1 refers specifically to yield line analysis as an acceptable approach.

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